ADDENDUM #1
TO CONB 2023-01
Neptune Beach Activity Center Porch and Exterior Finishes

February 9, 2023

To All Bid Holders and Prospective Respondents:

Thank you for submitting your question(s) to the City regarding CONB 2023-01. Please check https://www.nbfl.gov/home/pages/procurement for the bid documents and and addenda. The following are made a part of the original bid documents for CONB 2023-01:

1. I would like to request an electronic copy of the specifications or any other documents associated with this project: Project Title: NEPTUNE BEACH SENIOR ACTIVITY CENTER PORCH AND EXTERIOR FINISHES Bid Number: 2023-01 Posting Date: 1/12/2023
Response: See attached

2.

1. Per A3 on the elevations page what kind of screen is to be used in between the cmu piers?
2. Per A3 on the elevations page the roof is called out to be metal, but on the section A7 it shows the roof to be asphalt shingles please identify?
3. What is the ceiling type in the electrical room where the existing panel is located?
4. The fan and fixtures are stated to be selected by owner on A8, Will they be purchased by the owner?
5. In article 2 Procurement Contract Times, states that work shall be substantially completed by May 30th which is not obtainable in this current market. Can this date be pushed back.
6. Attachment #4, Pg. 2, Section 5.02.H states all items below must be maintained for (3) years after substantial completion. Typically, only the General Liability policy (and perhaps Umbrella) would have a requirement to remain in effect after completion. All of the other policies listed would not be required to be maintained for anytime frame after completion. Is this correct or should it just apply to General Liability (and perhaps Umbrella)?

Response:

1. Per A3 on the elevations page-the architect/design team suggested LATTICE between the cmu piers;
2. Per A3 on elevations page -the entire roof is METAL;
3. Ceiling inside the building is drop throughout;
4. The fans and fixtures will be bought and installed by the owner/City;
5. The Procurement Contract Times needs to be May 30, 2023, as funding sources have placed time restrictions on the monies. This can be renegotiated with the funder should

more time be required to complete the project. What do you believe to be a realistic timeline?

6. Attachment #4, Section 5.02.H regarding General Liability and/or Umbrella-I will have our City Attorney review this section and get back to you with his recommendation.

3. As stated on page 26 “The following documents are attached to and made a condition of this Bid”
Line Item 2. Section 00430 – Trench Safety Affidavit

This Section is Missing? – will you be issuing your own? – or would you like us to generate and provide our own c

Response: In reviewing our bid documents Table of Contents, Section 00430 (Trench Safety Affidavit) is not cited or included as part of the bid package. However, we would appreciate you including your own copy with your bid documents.

JACKSON GEOTECHNICAL ENGINEERING

Consulting Geotechnical Engineers

REPORT OF GEOTECHNICAL EXPLORATION
NEPTUNE BEACH SENIOR CENTER DRAINAGE IMPROVEMENTS
NEPTUNE BEACH, FLORIDA
JGE PROJECT NO. 22-246.1

Prepared for:

Marquis Latimer & Halback
34 Cordova Street
St. Augustine, Florida 32084

Prepared by:

Jackson Geotechnical Engineering
164 Plaza Del Rio Drive
St. Augustine, Florida 32084
Phone: 904-252-2292

April 8, 2022

JACKSON GEOTECHNICAL ENGINEERING

Consulting Geotechnical Engineers

April 8, 2022

Mr. Jeremy Marquis
Marquis Latimer & Halback
34 Cordova Street
St. Augustine, Florida 32084

Report of Geotechnical Exploration and Engineering Services
Neptune Beach Senior Center Drainage Improvements
Neptune Beach, Florida
JGE Project No. 22-246.1

Dear Mr. Marquis:

As requested, Jackson Geotechnical Engineering has completed a geotechnical exploration for the subject project. The exploration was performed to evaluate the general subsurface conditions at the location of the proposed stormwater pond, and to provide soil and groundwater parameters to facilitate retention pond design.

We appreciate this opportunity to be of service as your geotechnical consultant on this phase of the project. Please contact us if you have any questions, or if we may be of any further service.

Sincerely:
Jackson Geotechnical Engineering, LLC.

Jeff S. Jackson, P.E.
Licensed, Florida 51979

cc: Mr. Jeremy Calloway, P.E.
Maverick Engineering

Table of Contents

1.0 PROJECT INFORMATION

1.1 Site Location and Description

The project is located adjacent to the west side of Strickland Road, approximately 225 feet north of Forest Avenue, in Neptune Beach, Florida. The site is cleared, with vegetation consisting of grass. Based on visual observation, the subject area is relatively level.

1.2 Project Description

Project information was provided to us during correspondence with Mr. Jeremy Calloway, P.E. of Maverick Engineering. We were provided with an excerpt from a site plan that shows the layout of the proposed pond excavation, property boundaries, and adjacent roadways.

We understand the proposed project consists of excavating a retention pond at the site. The pond will be utilized for the collection and treatment of stormwater. It is expected the pond will be designed in accordance with dry retention criteria.

2.0 FIELD EXPLORATION

In order to explore the subsurface conditions within the area of the proposed stormwater pond, one auger boring (A-1) was performed to a depth of 6 feet below the ground surface. In addition, two permeability tests were performed on relatively undisturbed soil samples obtained from the location of the auger boring. The auger boring was located by measurement from existing site features. The location of the field testing, and the subsurface conditions encountered at the boring location, are presented in Appendix A on the Boring Location Plan and Subsurface Profiles, respectively.

3.0 LABORATORY TESTING

3.1 Index Testing

Soil samples recovered during the field exploration were visually classified in accordance with ASTM D 2488. The results of the classification testing are presented on the Subsurface Profile in Appendix A.

3.2 Permeability Testing

A horizontal and vertical permeability (hydraulic conductivity) test were conducted on the undisturbed soil samples to estimate the coefficient of horizontal permeability of the appropriate soil layers. The coefficient of permeability is a measure of a soil's ability to transmit water under hydraulic loading conditions. It typically is a required input parameter for groundwater modeling, such as dry pond recoveries, background seepage, etc. The laboratory permeability test is typically conducted by placing the undisturbed soil sample in a permeameter, and while in the permeameter, the soil sample is subjected to differential hydraulic loading over a period of time. The volume of water that is transmitted through the soil sample is recorded, and along with the known hydraulic

loading conditions, Darcy's law is utilized to calculate the coefficient of permeability. The coefficient of permeability is shown on the Subsurface Profile at the depth of which the soil samples were obtained.

4.0 GENERAL SUBSURFACE CONDITIONS

4.1 General Soil Profile

The boring location and general subsurface conditions that were encountered are presented on the Boring Location Plan and Subsurface Profile. When reviewing these records, it should be understood the soil conditions may change significantly at adjacent, unexplored locations. The following discussion summarizes the soil conditions encountered.

In general, the boring encountered fine sand (SP) throughout the 6-foot exploration depth. Three inches of topsoil was present at the boring location.

4.2 Groundwater Level

The groundwater level was measured at the boring location at a depth of 4.8 feet below existing grade. The depth of the groundwater level encountered at the boring location is presented on the Subsurface Profile.

The groundwater table will fluctuate depending on seasonal variations, adjacent construction, surface water runoff, etc. Our estimate of the normal seasonal high groundwater level at the boring location is presented on the Subsurface Profile in Appendix A. Our estimate is based on the results of the soil boring, review of available published literature, and information provided for this study. Should rainfall intensity exceed normal quantities, or should other variables that affect the seasonal high groundwater level be altered, the groundwater profile at the site could change significantly.

5.0 RETENTION POND RECOMMENDATIONS

5.1 General

The drainage system includes a dry retention pond. Dry ponds retain the necessary minimum amount of stormwater runoff (treatment volume) during the storm event. The volume retained is treated by infiltration into the ground. Infiltration into the ground is primarily affected by permeability of the soil, vertical height of stormwater stored in the pond (hydraulic loading), depth of the aquifer, soil porosity, and vertical distance between the pond bottom and the water table.

5.2 Retention Pond Modeling

The table below summarizes our recommendations for pond recovery modeling. A factor of safety of 2.0 should be utilized in the recovery analysis.

Location	Horizontal Permeability (ft/day)	Vertical Permeability (ft/day)	Effective Porosity	Depth to Bottom of Aquifer[1,2] (feet)	Estimated Depth of Seasonal High Groundwater Level[1] (feet)
A-1	30.0	26.3	25%	6	2.7

(1) Depth references ground surface at the time of the subsurface exploration.
(2) Aquifer depth limited to bottom of boring, in accordance with SJRWMD guidelines.

Note: Permeability values represent existing, in-situ soils. If fill is utilized, it should meet the specifications of the drainage engineer.

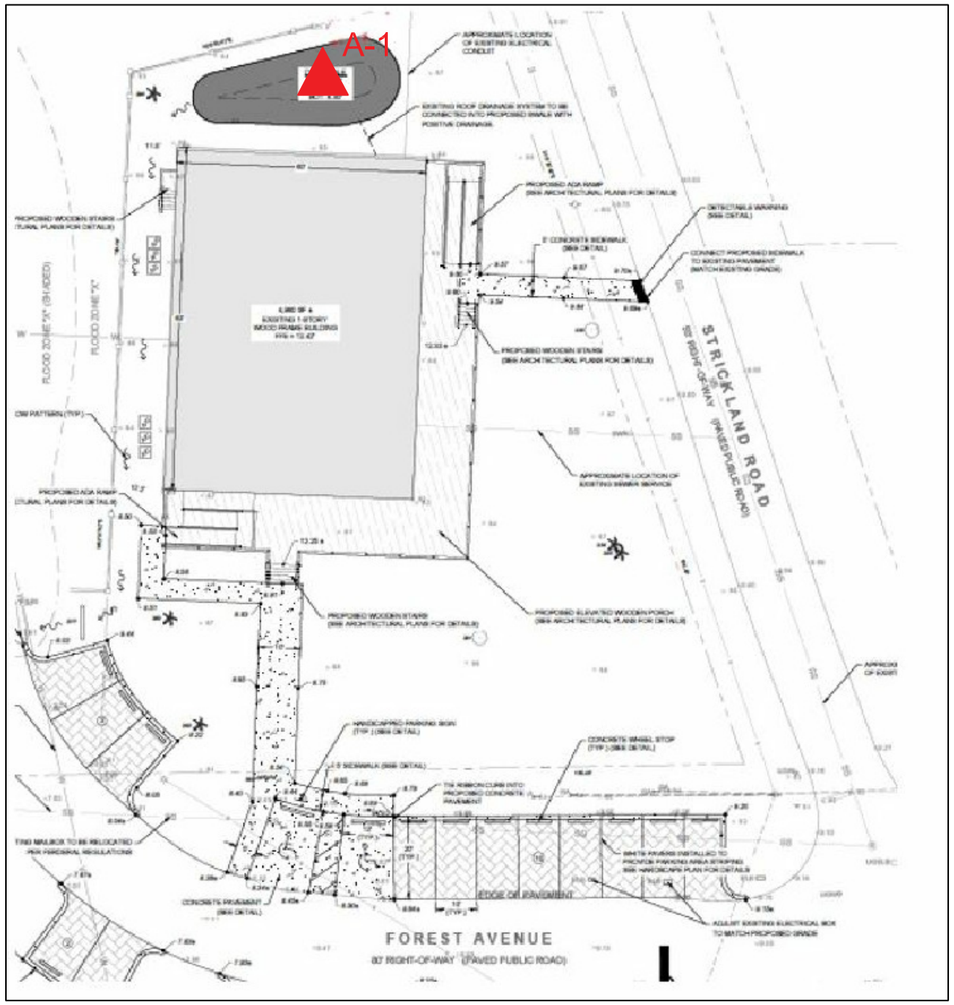
6.0 LIMITATIONS

We have conducted the geotechnical engineering in accordance with principles and practices normally accepted in the geotechnical engineering profession. Our analysis and recommendations are dependent on the information provided to us. Jackson Geotechnical Engineering is not responsible for independent conclusions or interpretations based on the information presented in this report.

APPENDIX A

BORING LOCATION PLAN

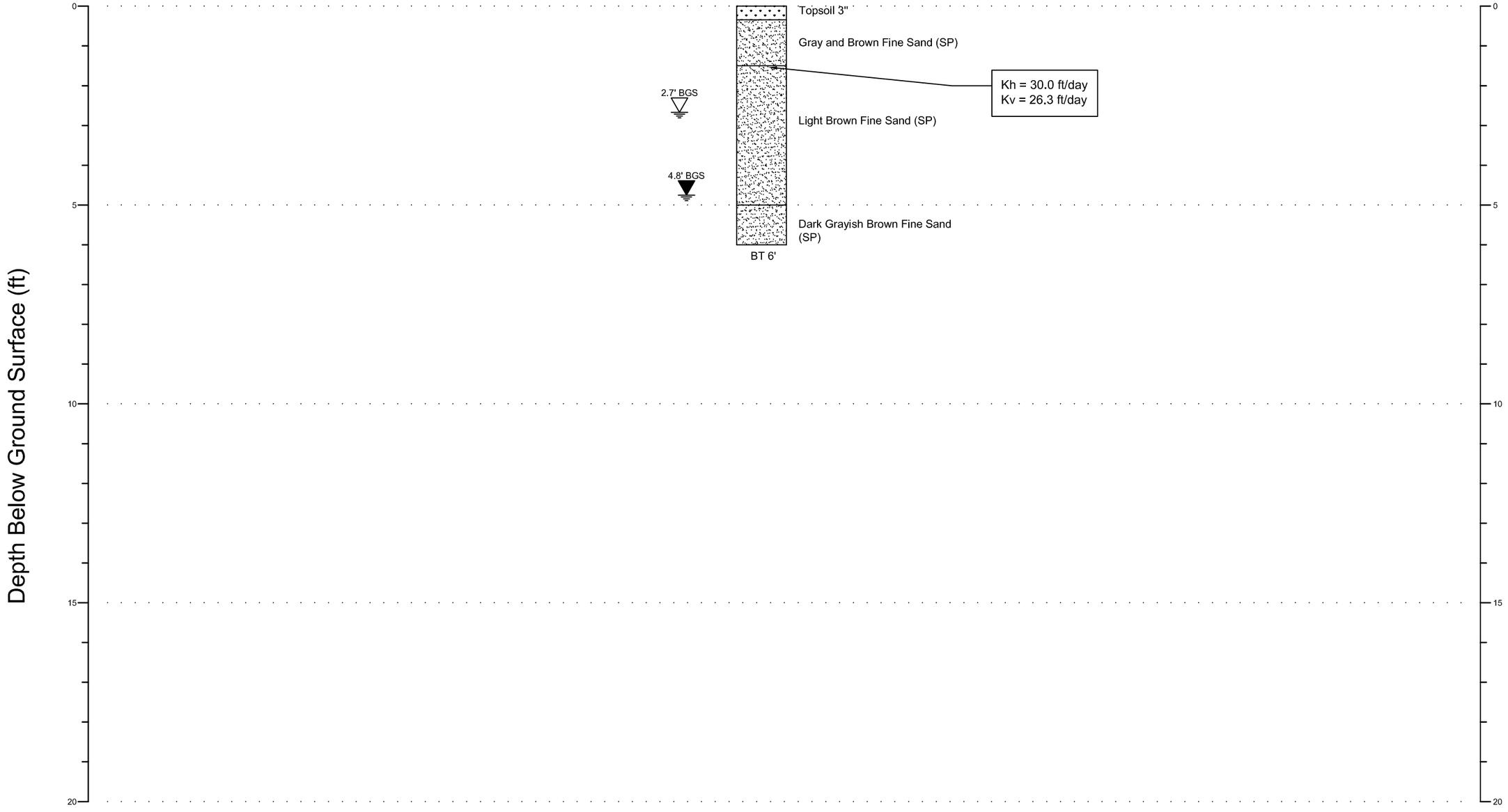
SUBSURFACE PROFILE



▲ Auger Boring Location

Jackson Geotechnical Engineering	
Boring Location Plan	
Neptune Beach Senior Center	
April 4, 2022	Drawn by: MJ
Project No. 22-246	Figure 1

A-1



Fine Sand (SP)

Topsoil

Groundwater Table Measured Below Ground Surface

Seasonal High Groundwater Table Estimated Below Ground Surface at Time of Exploration

Kh Coefficient of Horizontal Permeability

Kv Coefficient of Vertical Permeability

BT Boring Terminated

BGS Below Ground Surface

Jackson Geotechnical Engineering	
Subsurface Profiles	
Neptune Beach Senior Center	
April 4, 2022	Drawn by: MJ
Project No. 22-246	Figure 2

APPENDIX B

KEY TO SOIL CLASSIFICATION

FIELD AND LABORATORY TEST PROCEDURES

JACKSON GEOTECHNICAL ENGINEERING

Consulting Geotechnical Engineers

KEY TO SOIL CLASSIFICATION

CORRELATION OF PENETRATION WITH RELATIVE DENSITY & CONSISTENCY

SANDS AND GRAVEL	
BLOW COUNT	RELATIVE DENSITY
0-3	VERY LOOSE
4-10	LOOSE
11-30	MEDIUM DENSE
31-50	DENSE
OVER 50	VERY DENSE

SILTS AND CLAYS	
BLOW COUNT	CONSISTENCY
0-2	VERY SOFT
3-4	SOFT
5-8	FIRM
16-30	VERY STIFF
31-50	HARD
OVER 50	VERY HARD

PARTICLE SIZE IDENTIFICATION (UNIFIED CLASSIFICATION SYSTEM)

CATEGORY	*DIMENSIONS*
Boulders	Diameter exceeds 12 inches
Cobbles	3 to 12 inches
Gravel	Coarse – 0.75 to 3 inches in diameter Fine – 4.76 mm to 0.75 inch diameter
Sand	Coarse – 2.0 mm to 4.76 mm diameter Medium – 0.42 mm to 2.0 mm diameter Fine – 0.074 mm to 0.42 mm diameter
Silt and Clay	Less than 0.074 mm (invisible to the naked eye)

MODIFIERS

These modifiers provide our estimate of the amount of minor constituent (sand, silt, or clay size particles) in the soil sample

PERCENTAGE OF MINOR CONSTITUENT	*MODIFIERS*
0% to 5%	No Modifier
5 % to 12 %	With Silt, With Clay
12% to 30%	Silty, Clayey, Sandy
30% to 50%	Very Silty, Very Clayey, Very Sandy

APPROXIMATE CONTENT OF OTHER COMPONENTS (SHELL, GRAVEL, ETC.)	*MODIFIERS*	*APPROXIMATE CONTENT OF ORGANIC COMPONENTS*
0% to 5%	TRACE	1 to 2%
5% to 12%	FEW	2% to 4%
12% to 30%	SOME	4% to 8%
30% to 50%	MANY	>8%

FIELD AND LABORATORY TEST PROCEDURES

Auger Borings

The auger borings were performed using a continuous flight auger attached to a rotary drill rig or manually using a post-hole auger; and thus in general accordance with ASTM D 1452-80, "Soil Investigation and Sampling by Auger Borings". Representative samples of the soils brought to the ground surface by the augering process were placed in watertight containers and sealed. After completing the drilling operations, the samples for each boring were transported to the laboratory where the Geotechnical Engineer examined them in order to verify the driller's field classifications. The samples will be kept in our laboratory for a period of two months after submittal of formal written report, unless otherwise directed by the Client.

Soil Classification

Soil samples obtained from the performance of the borings were transported to our laboratory for observation and review. An engineer, registered in the State of Florida and familiar with local geological conditions, conducted the review and classified the soils in accordance with ASTM 2488. The results of the soil classification are presented on the boring records.

Constant Head Permeability Test

The coefficient of permeability for the laminar flow of water through granular soils was determined in general accordance with the latest revision of ASTM D 2434. The constant head permeability test is a measure of the quantity of water that flows through a sample contained in a cylinder of known height and diameter in a measured time while maintaining a constant head of water on the sample. The coefficient of permeability is determined by application of the Darcy's Law shown below:

$$k = \frac{Q\,L}{hAt}$$

k = Coefficient of permeability

Q = Quantity of water discharge

L = Length of specimen

h = Constant head of water

A = Cross-sectional area of specimen

t = Total time of discharge

Undisturbed Sampling

Relatively undisturbed samples were obtained in general accordance with the latest revision of ASTM A 1587, "Thin-Walled Tube Sampling of Soils". Manual methods were used to advance the 3-inch O.D. – 16 gauge stainless steel sampler tubes into the soils at the selected depths. After retrieving the samples, the ends were capped and then transported to our laboratory.

BUILDING FOOTAGE INFORMATION

NEW DECK ——— 1614 Sq. Ft.
NEW H.C. RAMPS ——— 474 Sq. Ft.

TYPE OF CONSTRUCTION

TYPE VB
PROJECT IS LOCATED EAST OF I-95 WIND-BORNE DEBRIS PROTECTION IS REQUIRED

WIND ZONE INFORMATION

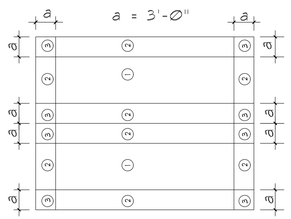
THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH, & MEETS THE REQUIREMENTS OF THE FLORIDA BUILDING CODE 2020 7th EDITION

1. BASIC WIND SPEED (3 SECOND GUSTS) ——— 130 M.P.H.
2. WIND IMPORTANCE FACTOR ——— 1.0
3. WIND EXPOSURE CATEGORY ——— B
4. INTERNAL PRESSURE COEFFICIENT +.18 OR -.18
5. COMPONENT & CLADDING WIND LOADS Lbs/Sq.FT.

HEIGHT AND EXPOSURE ADJUSTMENT COEFFICIENTS = 1.00

	EFFECTIVE WIND AREA Ft. Sq.							
	10		20		50		100	
ROOF								
1	10.5	-25.9	10.0	-25.2	10.0	-24.4	10.0	-23.7
2&3	10.5	-43.5	10.0	-38.8	10.0	-32.7	10.0	-28.1
WALL								
4	25.9	-28.1	24.7	-26.9	23.2	-25.4	22.0	-24.2
5	25.9	-34.7	24.7	-32.4	23.2	-29.3	22.0	-26.9

ROOF OVERHANG	10	20	50	100
	-37.3	-36.7	-35.2	-35.1
	-61.5	-48.3	-30.8	-17.6

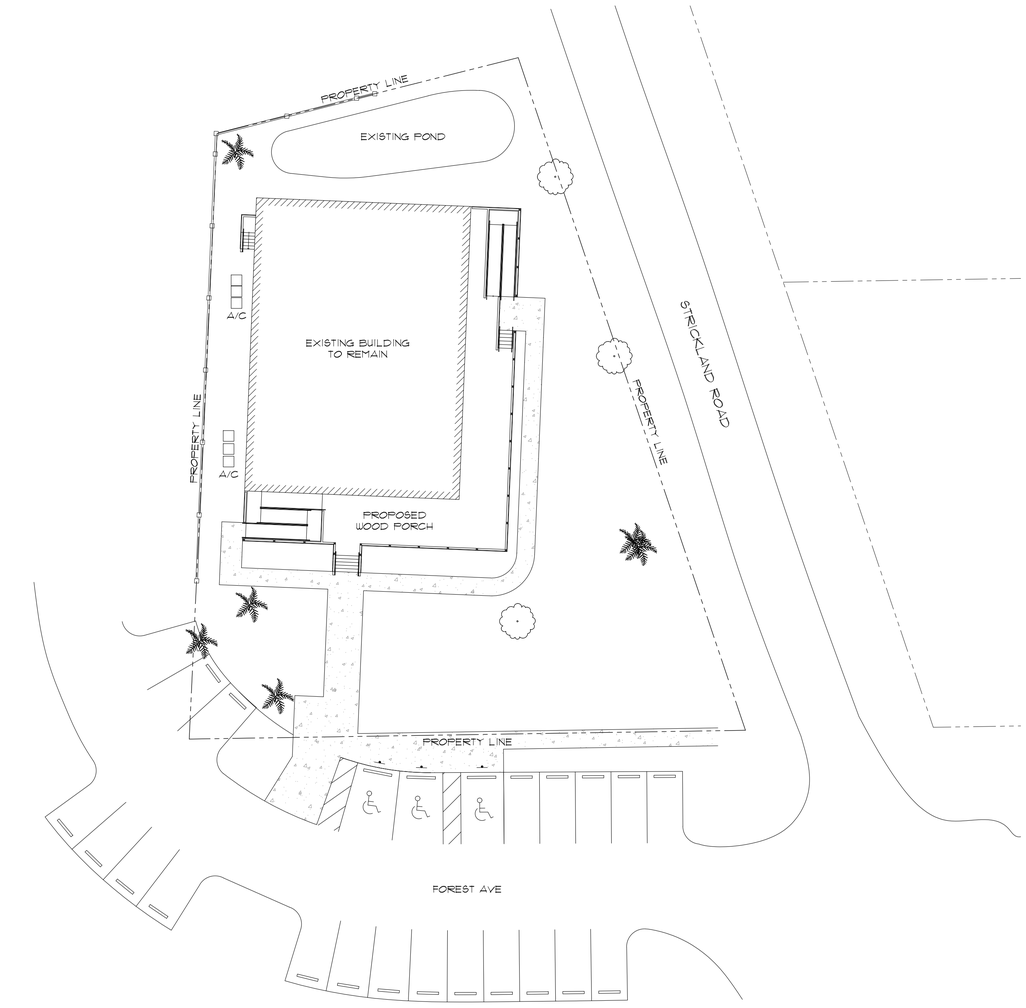


NOTES:

1. FOR EFFECTIVE AREAS BETWEEN THOSE GIVEN ABOVE THE LOAD MAY BE INTERPOLATED BY THE DESIGNER, OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE AREA.
2. SEE FIGURES FOR LOCATION OF ZONES.
3. PLUS AND MINUS SIGNS SIGNIFY PRESSURE ACTING TOWARD AND AWAY FROM THE BUILDING SURFACES.

CODE ANALYSIS

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH, & MEETS THE REQUIREMENTS OF:
BUILDING - FLORIDA BUILDING CODE, BUILDING 2020 7th EDITION
PLUMBING - FLORIDA BUILDING CODE, PLUMBING 2020 7th EDITION
MECHANICAL - FLORIDA BUILDING CODE, MECHANICAL 2020 7th EDITION
LIFE SAFETY - NATIONAL FIRE PREVENTION ASSOC. CODE, LATEST EDITION
FIRE CODE - FLORIDA FIRE PREVENTION CODE 7th EDITION
HANDI-CAP CODE - FLORIDA BUILDING CODE, CHAPTER 11 - ACCESSIBILITY, 2020 7th EDITION
ACCESSIBILITY CODE - FLORIDA BUILDING CODE, CHAPTER 11 - ACCESSIBILITY, 2020 7th EDITION
ENERGY CODE - FLORIDA BUILDING CODE, ENERGY CONSERVATION, 2020 7th EDITION
NATIONAL ELECTRIC CODE CURRENT EDITION



N
Site Plan
1"=20'-0"

Neptune Beach
Senior Community Center
Neptune Beach, Florida

INDEX OF DRAWINGS

STRUCTURAL NOTES

1. DESIGN LOADS:
   A. ROOF LIVE LOADS ——— 16 p.s.f.
   B. FLOOR LIVE LOADS ——— 40 p.s.f.
   C. WIND LOADS ——— 130 m.p.h.
   (FL. BLDG. CODE 2020)
2. MATERIAL
A. CONCRETE: DESIGN AND CONSTRUCTED PER A.C.I. 318-83

ITEM	COMPRESSIVE STRENGTH @ 28 DAYS
SLAB	2500 P.S.I.
C.M.U. FILLED CELLS & BEAMS	2500 P.S.I.

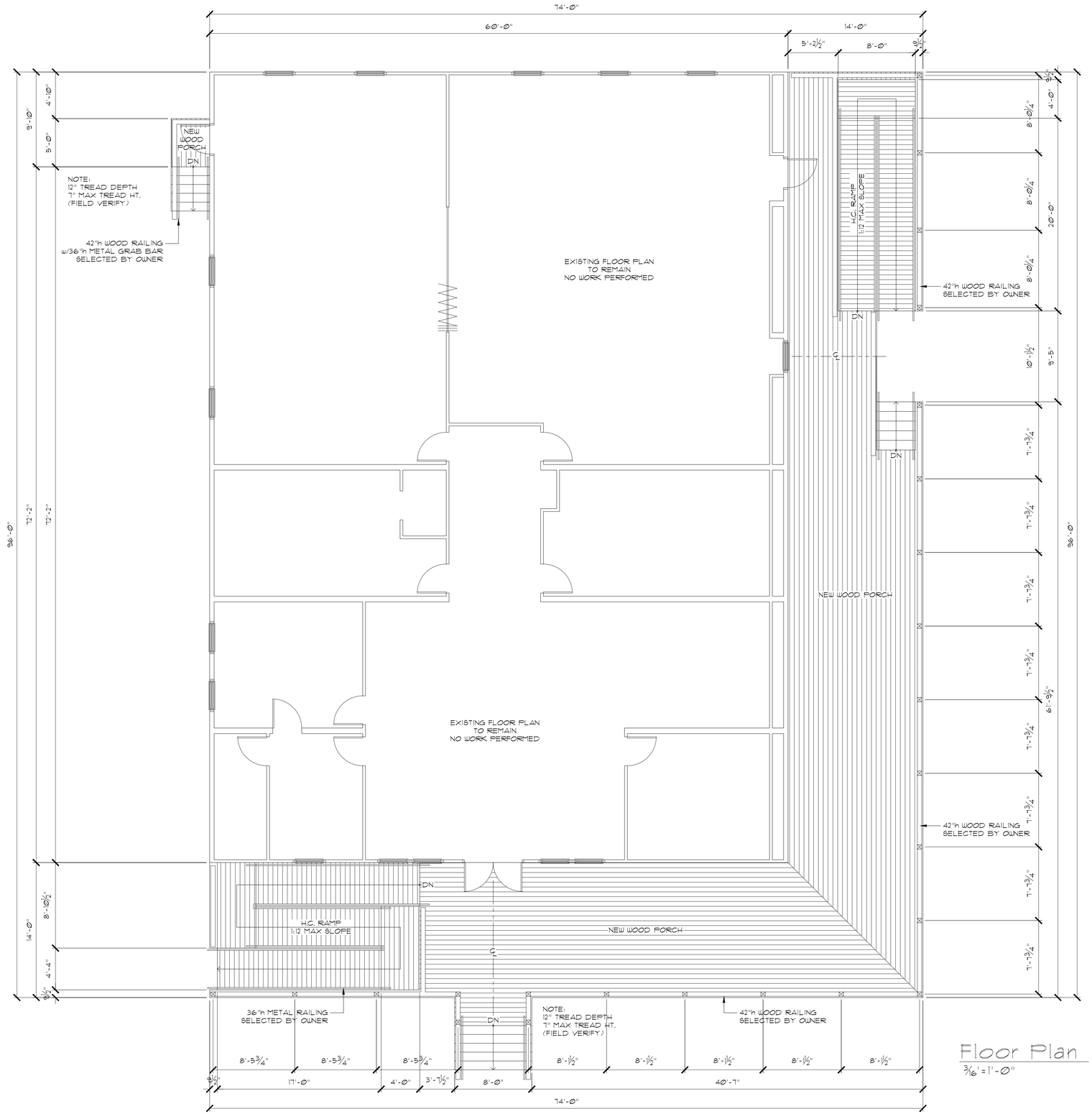
B. REINFORCING STEEL: CONFORM TO ASTM A-615 GAGE 60
C. STRUCTURAL STEEL: DESIGN PER CURRENT ADDITION OF A.I.S.C.
   1. SHAPES AND PLATES CONFORM TO ASTM A-36
   2. WELDING CONFORM TO "AWS D1.1, STRUCTURAL WELDING CODE"
   3. ANCHOR BOLTS AND STEEL TO WOOD SHALL CONFORM TO ASTM A-307
   4. WELDED CONNECTIONS NOT SHOWN ON DRAWING SHALL HAVE ALL CONTACTING STEEL SURFACES CONTINUOUS WELDED WITH SUFFICIENT WELD TO FULLY DEVELOP THE THINNER MATERIAL.
D. FRAMING LUMBER: SOUTHERN PINE PER N.F.P.A., NATIONAL DESIGN SPECS. FOR WOOD CONSTRUCTION.
   1. SAWN LUMBER 2x4 THRU 2x12 SHALL BE SOUTHERN PINE NO. 2 @ 19% M.C.
   2. INTERIOR WALL STUDS SHALL BE SPRUCE-PINE-FIR NO.2
   3. L.V.L. BEAMS SHALL BE SOUTHERN PINE Fb=2400 P.S.I.
   4. SAWN LUMBER 4x4 AND LARGER SHALL BE SOUTHERN PINE NO. 1 @ 19% M.C.
E. WOOD FLOOR & ROOF TRUSSES: DESIGN BY THE MANUFACTURER TO SUPPORT DEAD, WIND AND LIVE LOADS.
   1. MANUFACTURE SHALL SUBMIT ERECTION DRAWINGS FOR REVIEW BEFORE FABRICATING TRUSSES.
   2. ERECTION DRAWINGS SHALL SHOW ALL LATERAL AND DIAGONAL BRACING AS REQUIRED IN THE TRUSS SYSTEM.
   3. TRUSS TO TRUSS CONNECTIONS SHALL BE DESIGNED BY THE MANUFACTURER.
F. PLYWOOD ROOF AND WALL SHEATHING:
   CONFORM TO THE AMERICAN PLYWOOD ASSOC. STANDARDS AND SHALL BE AP C-D INT. WITH EXTERIOR GLUE (CDX) MIN.
G. CONCRETE MASONRY UNITS: CONFORM TO ASTM C-90.
   MORTAR SHALL BE TYPE M OR S.
H. WOOD FRAMING ANCHORS AND HURRICANE TIE CLIPS
   SHALL BE "GO-BOLTS" OR EQUAL.
4. CONCRETE MASONRY UNITS:
   A. ALL C.M.U. SHALL HAVE #5 BAR VERTICAL WITH CELL FILLED WITH CONCRETE AS SHOWN ON DRAWINGS.
   B. ALL C.M.U. SHALL HAVE HORIZONTAL JOINT REINFORCING SPACED 16" O.C. VERTICAL. REINFORCING SHALL BE FABRICATED FROM 9 GUAGE GALVANIZED WIRE.
5. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TEMP. BRACING UNTIL THE ENTIRE STRUCTURE IS PLUMB AND SECURED IN PLACE.
6. SHEATHING NAILING:
   A. ROOF SHEATHING SHALL BE NAILED AS FOLLOWS:
      8d RING SHANK NAILS
      8d NAILS @ 6" O.C. AT PANEL EDGES.
      8d NAILS @ 9" O.C. AT ALL INTERMEDIATE SUPPORTS.
      8d NAILS @ 4" O.C. AT ALL SUPPORTS WITHIN 4'-0" OF EDGES.
   B. PORCH CEILING OR SUB CEILING WITH RING SHANK NAILS:
      8d NAILS @ 4" O.C. AT PANEL EDGES.
      8d NAILS @ 8" O.C. AT ALL INTERMEDIATE SUPPORTS.
   C. ALL EXTERIOR WALLS BETWEEN OPENINGS AND AT CORNERS SHALL BE SHEAR WALL SEGMENTS.
      PLY-WOOD NAILING TO BE: 8d NAILS @ 6" O.C. EACH SHEAR WALL SEGMENT SHOULD HAVE 1/2 Ø THREADED ROD WITHIN 8" OF SHEAR WALL.
7. C.M.U. WALL OPENING HEADS, JAMBS, AND WINDOW SILLS SHALL BE 2x6 MIN. P.T. WITH 1/4" DIA. x 3 1/4" LONG "TAPCONS" @ 18" O.C.
8. ALL EXTERIOR WINDOWS AND DOORS SHALL MEET 130 M.P.H. WIND SPEED. WIND BORNE DEBRIS PROTECTION REQUIRED: IMPACT RESISTANT WINDOWS OR SHUTTERS BY ARCHITECT OR CONTRACTOR.

LES THOMAS
ARCHITECT
32 CORDOVA ST., ST. AUGUSTINE, FLORIDA 824-9508

NEW PORCH
NEPTUNE BEACH COMMUNITY CENTER
NEPTUNE BEACH, FLORIDA

Date:	4.29.22
Scale:	AS NOTED
Drawn:	MURPHY
File:	NEPTUNE

SHEET
A1



Floor Plan

3/16" = 1'-0"

NOTE:
CONTRACTOR/BUILDER
SHALL FIELD VERIFY
ALL DIMENSIONS AND CONDITIONS
PRIOR TO CONSTRUCTION.

LES THOMAS
A R C H I T E C T
32 CORDOVA ST., ST. AUGUSTINE, FLORIDA 824-9508

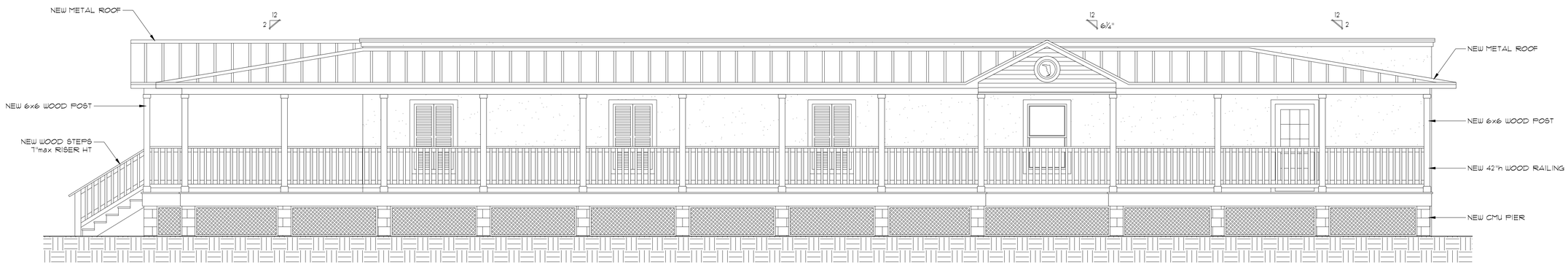
NEW PORCH
NEPTUNE BEACH COMMUNITY CENTER
NEPTUNE BEACH, FLORIDA

Date:	4.29.22
Scale:	AS NOTED
Drawn:	MURPHY
File:	NEPTUNE

SHEET
A2

Front Elevation

1/4"=1'-0" North



Right Elevation

1/4"=1'-0" East

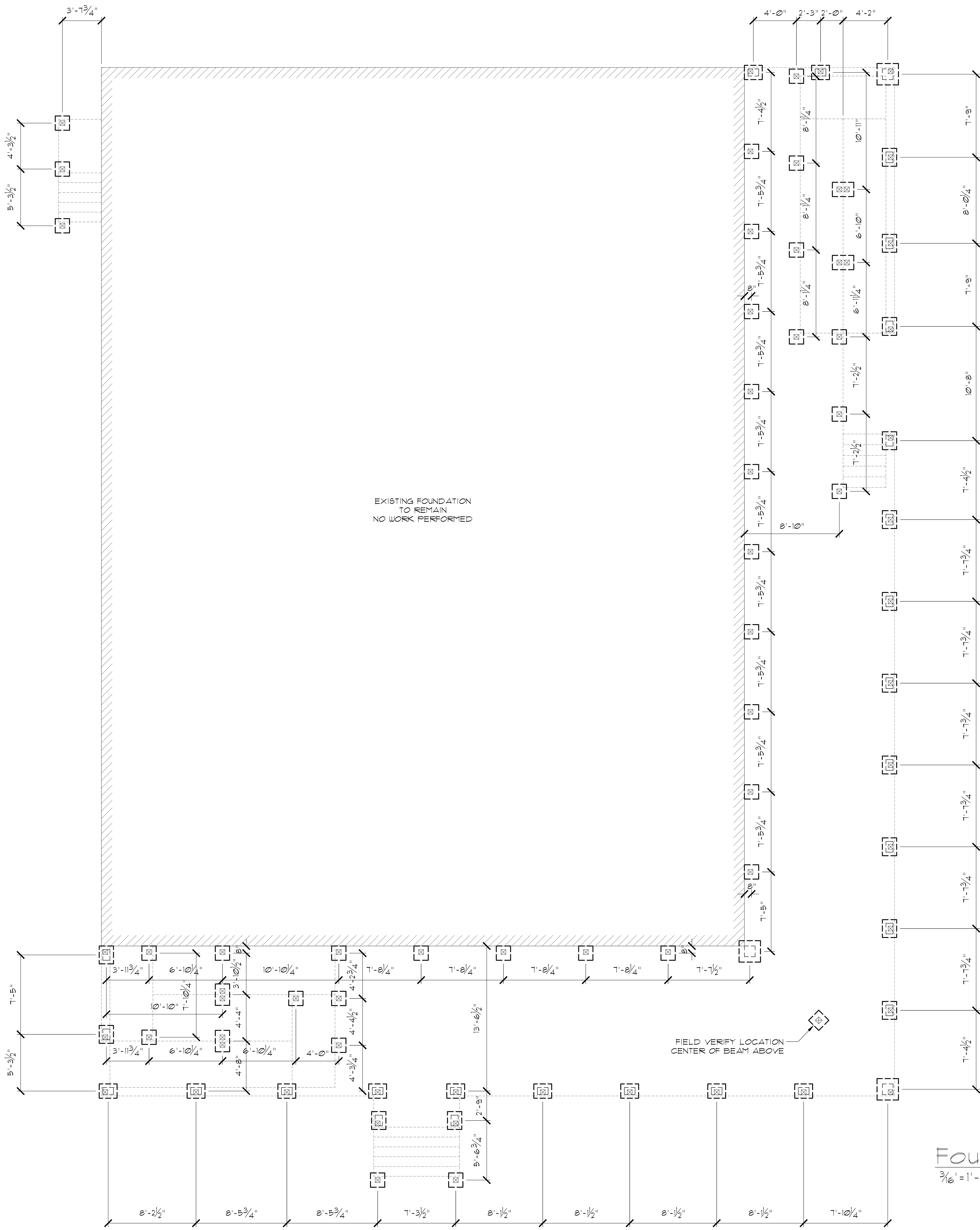
NOTE:
ELEVATION DRAWINGS ARE INTENDED FOR DESIGN PURPOSES ONLY. CONTRACTOR/BUILDER SHALL FIELD VERIFY ALL DIMENSIONS AND CONDITIONS PRIOR TO CONSTRUCTION...

LES THOMAS
ARCHITECT
32 CORDOVA ST., ST. AUGUSTINE, FLORIDA 824-9508

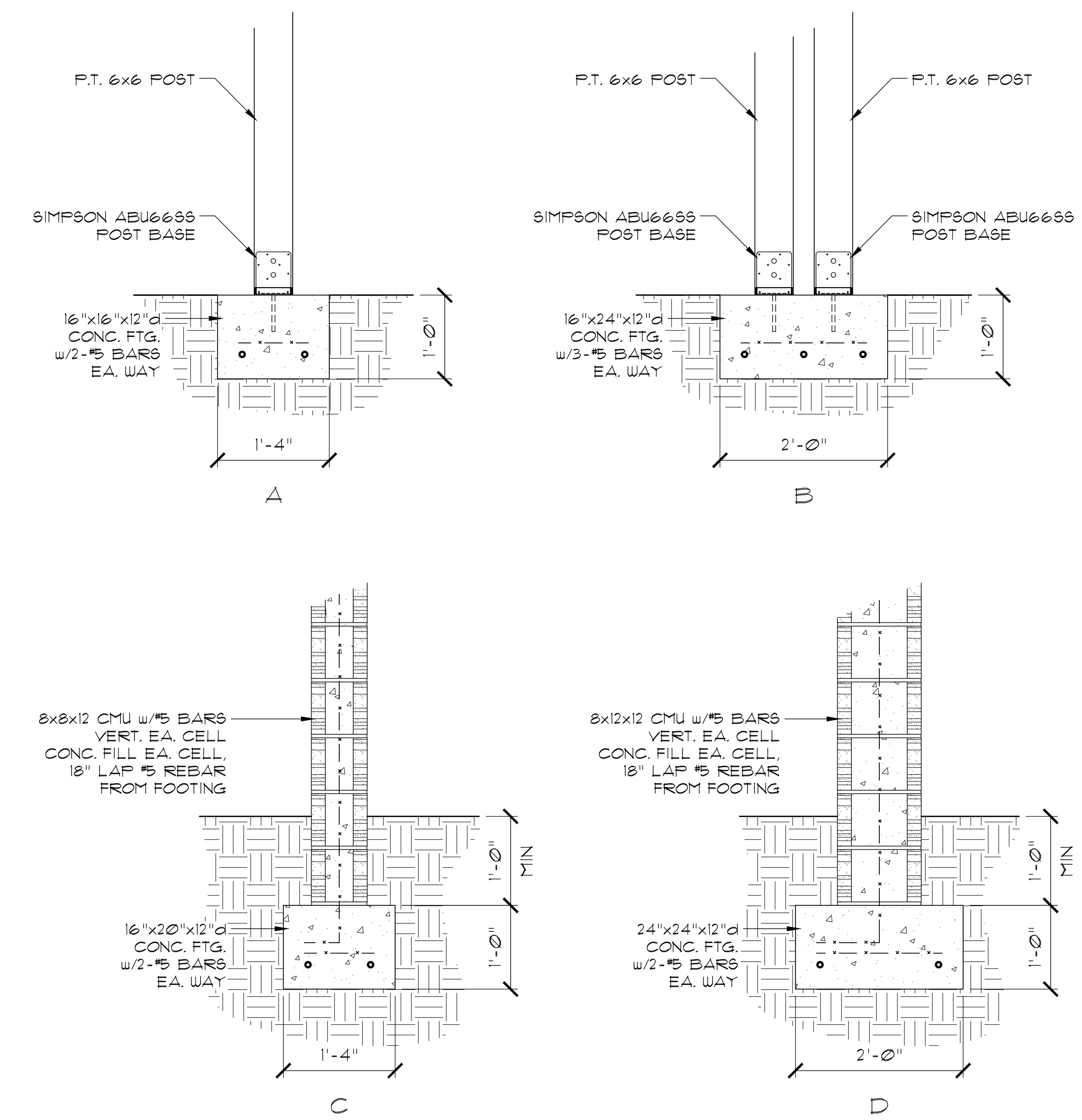
NEW PORCH
NEPTUNE BEACH COMMUNITY CENTER
NEPTUNE BEACH, FLORIDA

Date:	4.29.22
Scale:	AS NOTED
Drawn:	MURPHY
File:	NEPTUNE

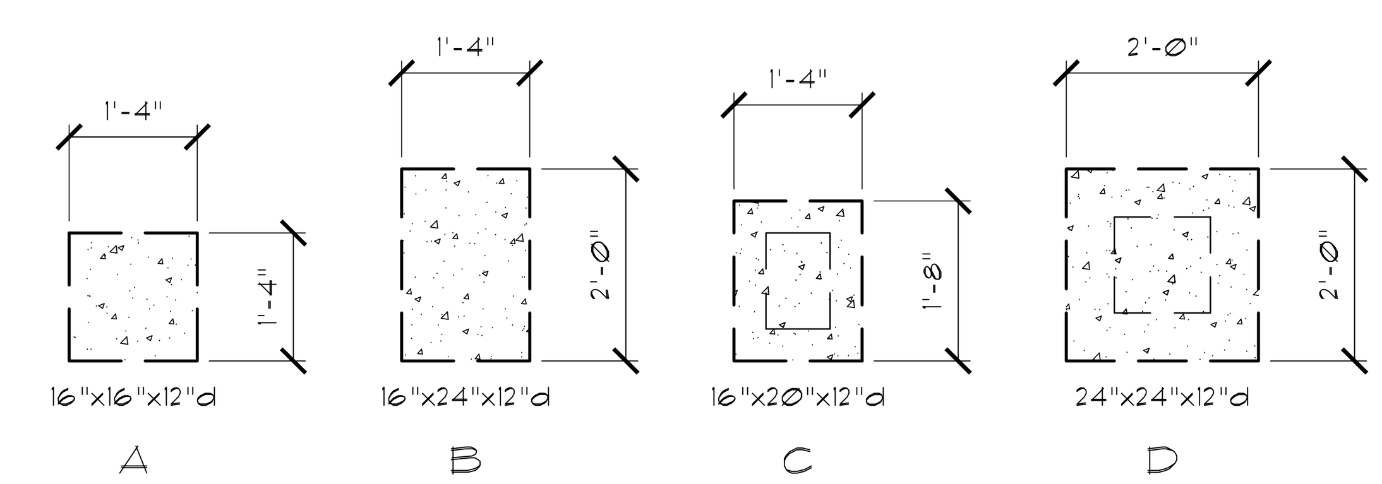
SHEET
A3



Foundation Plan
3/16" = 1'-0"



Foundation Details
1/2" = 1'-0"



Footing Types
1/2" = 1'-0"

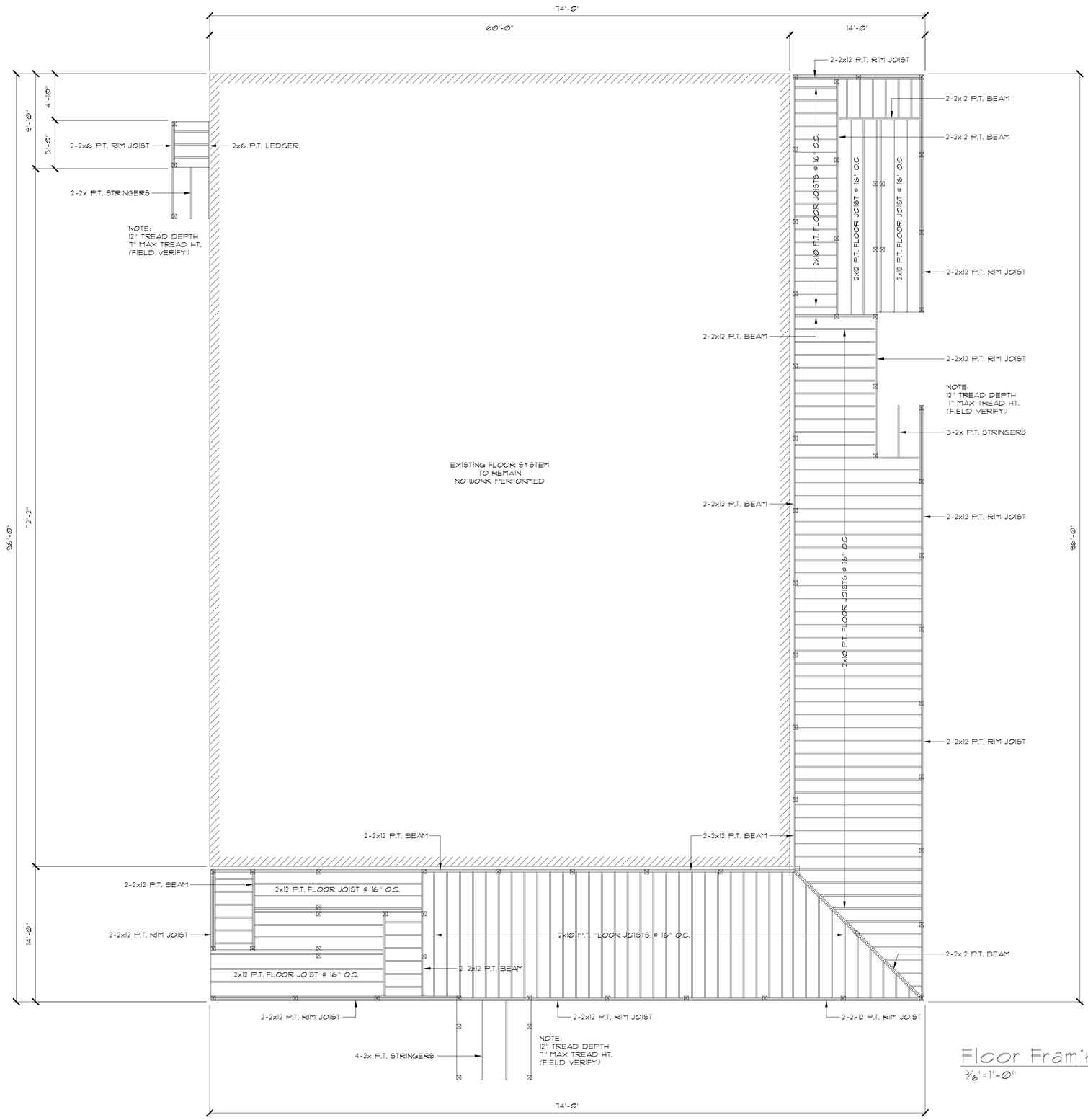
NOTE:
CONTRACTOR/BUILDER SHALL FIELD VERIFY ALL DIMENSIONS AND CONDITIONS PRIOR TO CONSTRUCTION.

LES THOMAS
ARCHITECT
32 CORDOVA ST., ST. AUGUSTINE, FLORIDA 824-9508

NEW PORCH
NEPTUNE BEACH COMMUNITY CENTER
NEPTUNE BEACH, FLORIDA

Date: 4.29.22
Scale: AS NOTED
Drawn: MURPHY
File: NEPTUNE

SHEET
A4



Floor Framing Plan

3/16" = 1'-0"

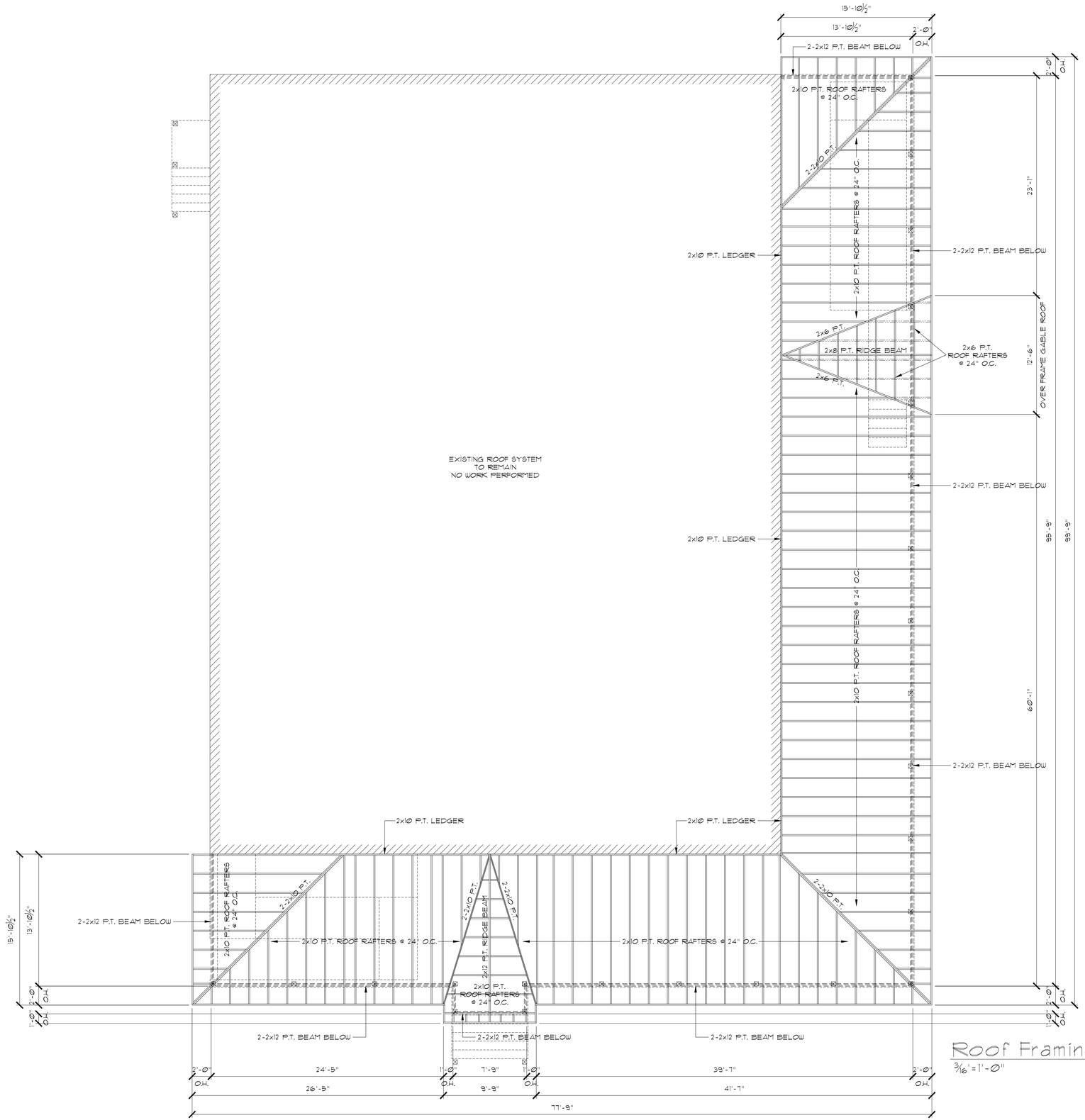
NOTE:
CONTRACTOR/BUILDER
SHALL FIELD VERIFY
ALL DIMENSIONS AND CONDITIONS
PRIOR TO CONSTRUCTION.

LES THOMAS
ARCHITECT
32 CORDOVA ST., ST. AUGUSTINE, FLORIDA 824-9508

NEW PORCH
NEPTUNE BEACH COMMUNITY CENTER
NEPTUNE BEACH, FLORIDA

Date:	4.29.22
Scale:	AS NOTED
Drawn:	MURPHY
File:	NEPTUNE

SHEET
A5



Roof Framing Plan

3/16" = 1'-0"

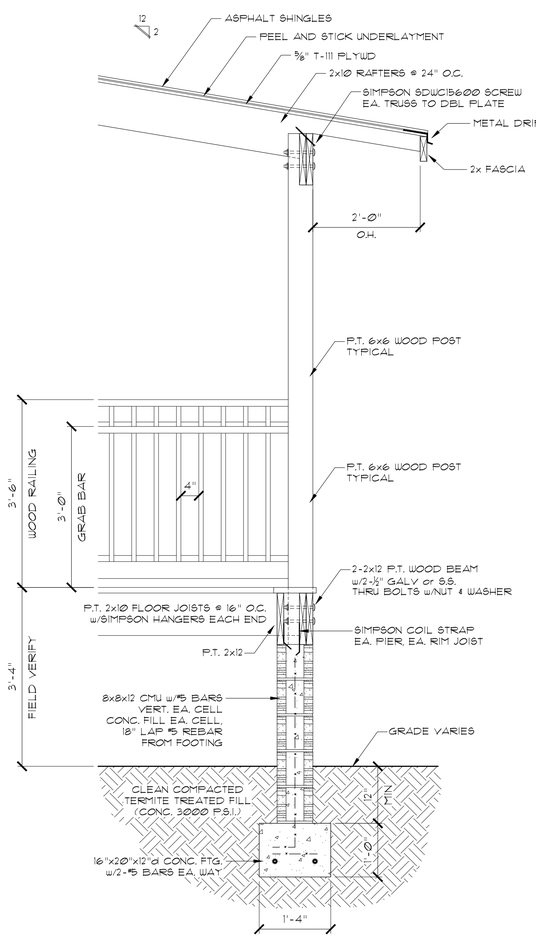
NOTE:
CONTRACTOR/BUILDER
SHALL FIELD VERIFY
ALL DIMENSIONS AND CONDITIONS
PRIOR TO CONSTRUCTION.

LES THOMAS
ARCHITECT
32 CORDOVA ST., ST. AUGUSTINE, FLORIDA 824-9508

NEW PORCH
NEPTUNE BEACH COMMUNITY CENTER
NEPTUNE BEACH, FLORIDA

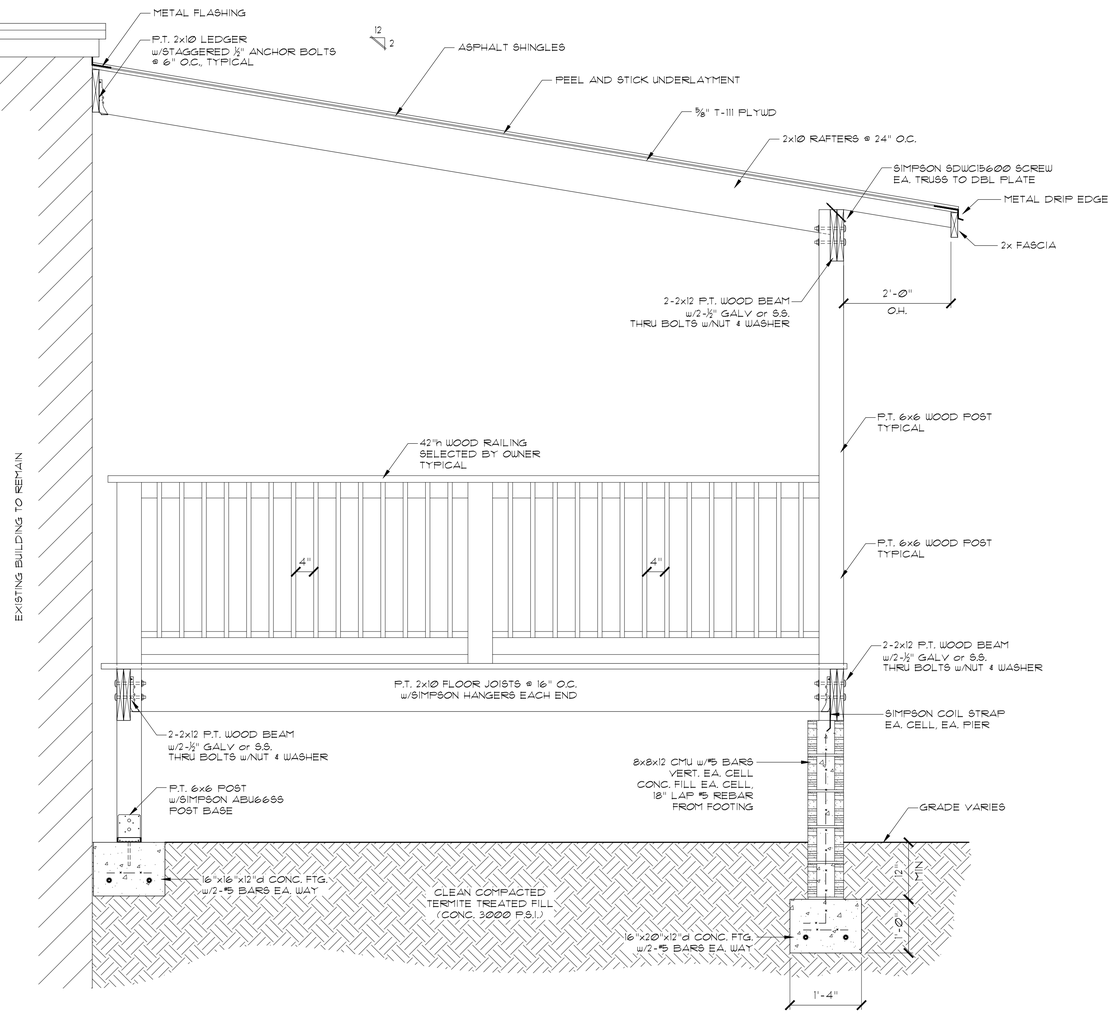
Date:	4.29.22
Scale:	AS NOTED
Drawn:	MURPHY
File:	NEPTUNE

SHEET
A6



Ramp Section

3/4" = 1'-0"



Typ Section

3/4" = 1'-0"

NOTE:
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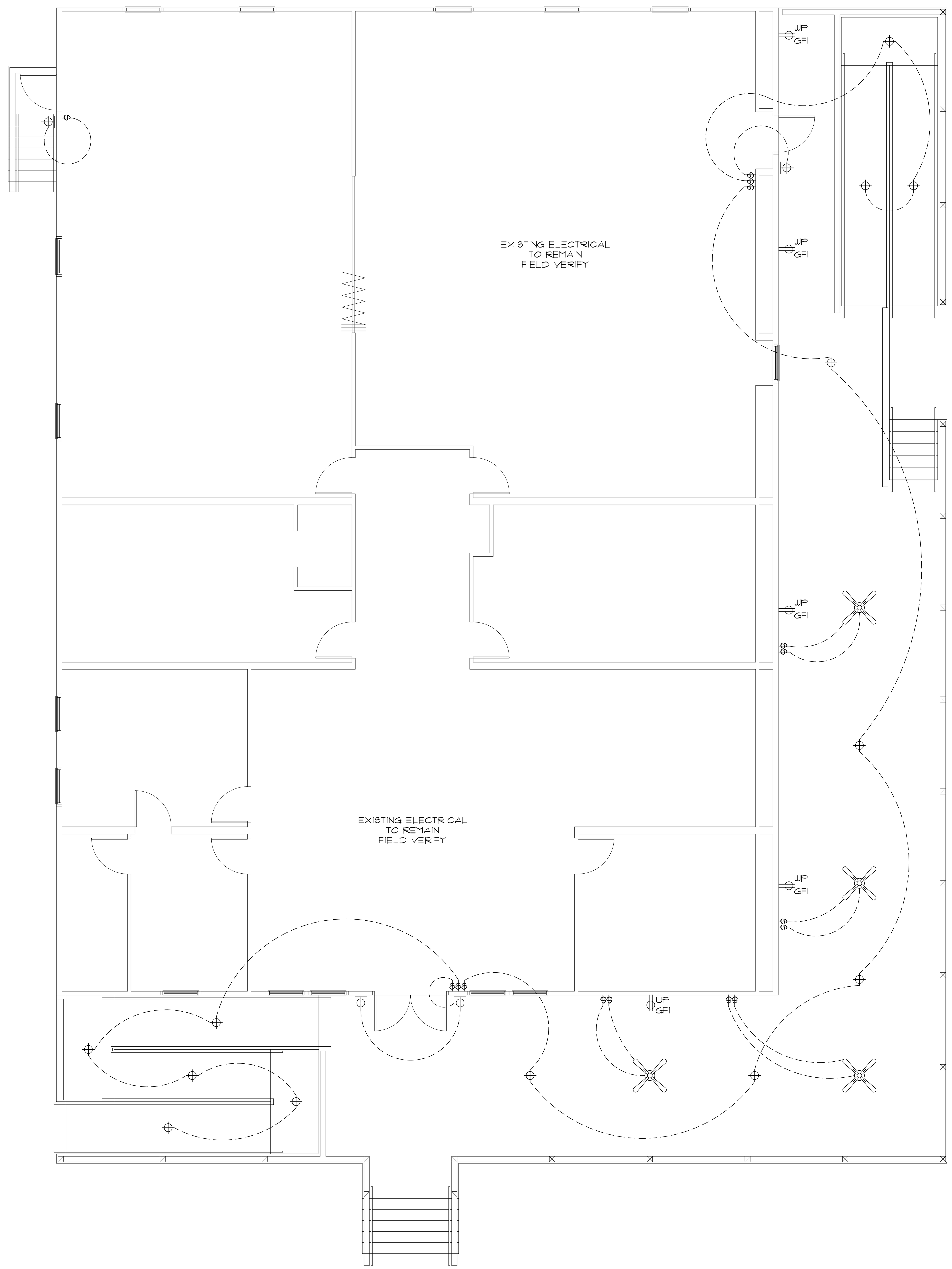
Date:	4.29.22
Scale:	AS NOTED
Drawn:	MURPHY
File:	NEPTUNE

SHEET
A7



Electrical Legend

(receptacle symbol)	120V WALL MOUNT DUPLEX RECEPTACLE 12" AFF. U.N.O.
(wall light symbol)	WALL MTD LIGHT, SELECTED BY OWNER
(ceiling light symbol)	CEILING MTD LIGHT, SELECTED BY OWNER
$	WALL MOUNTED SWITCH - 48" AFF. U.N.O
GFI	w/ GROUND FAULT INTERRUPTER
WP	WEATHER PROTECTED
(fan symbol)	CEILING MTD FAN WITH LIGHT, SELECTED BY OWNER

Electrical Plan

3/16" = 1'-0"

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Drawn: MURPHY
File: NEPTUNE

SHEET
A8